



FLOOR FALSEWORK DESIGN CALCULATION

SYSTEM: CUP LOCK SYSTEM

PROJECT:

PROPOSED MOSQUE (G+1) & SERVICE BLOCK AT WADI AL SAFA 5, DUBAI, U.A.E.

CLIENT:

MR. ABDUL GAFFAR SYED MOHD AL HASHIMI

CONSULTANT:

CITY SPACE ENGINEERING CONSULTANCY.

MAIN CONTRACTOR:

ARIFCO BUILDING CONTRACTING L.L.C.

REFERENCE DWG.NO.:

DBC/H/2020/6501 (ROOF SLAB)

PREPARED BY:

ENGR. A.MANALANSAN

CHECKED BY:

ENGR. B. GUNABE



DATE: 29/03/2020

PROJECT : PROPOSED MOSQUE (G+1) & SERVICE BLOCK AT WADI AL SAFA 5, DUBAI, U.A

REFERENCE DWG NO.: DBC/H/2020/6501 (ROOF SLAB)

SLAB DEPTH 300mm

A. Slab Falsework

Cuplock Support System- (Grid 1.800x1.30)

Slab Falsework

| | | |
|-------------------------|----------------------------|--------------------------------|
| Self weight of Concrete | = | 25.0 kN/m ³ |
| Self weight of Formwork | = | 0.50 kN/M ² |
| Live load | = | 1.50 kN/M ² |
| Concrete | .30x25.0 kN/m ² | = 7.50 kN/M ² |
| Construction live load | = | 1.50 kN/M ² |
| | q | = 9.00 kN/M² |

B. Plywood Sheathing

| | | |
|-------------------|---|------------------------|
| Plywood thickness | = | 18 mm |
| Normal Size | = | 1220 x 2440 mm |
| Concrete Pressure | = | 9.00 kN/M ² |

See Deflection Diagram for 18 mm Plywood

| | | |
|--|---|---------------------------------|
| Max. Allowable Spacing of Spacing of Support | = | 575 mm (allow.) |
| Actual / Assembly Spacing of Support | = | 400 mm < allowable OK |

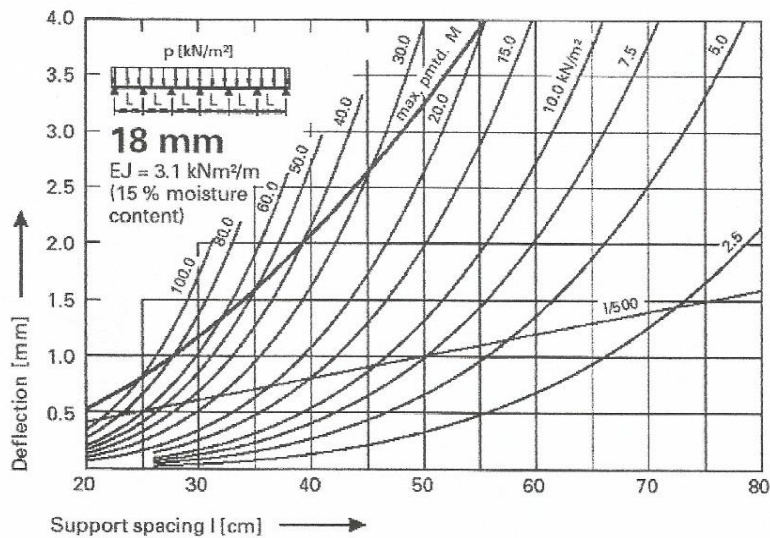


DESARCH
SCAFFOLDING

Diagram for 18mm Plywood according to DIN 18 218

For the concrete pressure of 14.0 kN/m^2
Maximum allowable support spacing 520 mm
Maximum allowable deflection 2.0 mm

Deflection diagram Finnish birch plywood



Since the actual spacing is 450 mm which is below to allowable 520mm.
Therefore, the plywood is safe.



C. Check Secondary beam

| | |
|--|-------------------------------|
| Technical Information { see reference page-) | |
| Infill beam | |
| length Actual= 1.20 m | Length Normal = 1.30 m |
| Safe Distributed load= 7.50 kN/ M² | |

Imposed load = 9.00 kN/m³

Secondary beam (infill Beam) Spacing = 400 mm

Loading Calculation

w= imposed load x secondary beam spacing
w = 9.00 kN/ M² x 0.400 = 3.60 kN/ m

Since the actual load is 3.60 kN/ m x 1.20 = 4.32 kN < 7.50 kN so it's OK.

Therefore, Secondary beam is safe.

D. Check Primary beam

| | |
|---|-------------------------------|
| Technical Information { see reference page-) | |
| Decking beam | |
| length Actual= 1.65m | Length Normal = 1.80 m |
| Safe Distributed load= 40.00 kN/ M² | |

Loading Calculation

w= imposed load x primary beam spacing
w = 9.00 kN/ M² x 1.80 x 1.30 = 21.06 kN/ m

Since the actual load is 21.60 kN which is below the allowable load **40kN**

Therefore, Primary beam is safe.



E. **Support System**

(Combination of Standard, ledgers, Drop head, Base Jack, Universal Jack & Bracings)

Technical Information

According to Cuplock Technical Data Information

allowable load per Standard with lift of ledger 1.5m

and providing bracing every after 2 bays at both directions is 35 kN

Loading Calculation

| | | |
|---------------------------|---|--------------------------------|
| Area of Slab per standard | = | 1.80x1.30= 2.34 M ² |
| Self weight of Decking | = | 0.50 kN/M ² |
| Load on per Standard | = | 9.00+0.50 *2.34=22.23 kN |

Therefore Support is safe. OK

Ledger Spacing

According to BS 1139-1:1990, see table B.2 reference

Maximum permissible axial load for unbraced used tubes with effective length of 1500mm is 35.0 kN is greater than allowable.



F. Diagonal Bracing

(Diagonal bracing with scaffold tube 48.30 mm Dia. & swivel coupler)

The design of bracing to horizontal resistance force required to be transmitted is specifies in BS5975:1996

Code of practice for falsework

The BS code specifies minimum lateral stability criteria aqualine to the greater of either 2.50% of the vertical load in the standards acting horizontal forces from wind erection tolerances, non vertical and concrete pressure of other forces acting as described in the code.

1. Horizontal force equavalent to 2.50% of vertical loa (Hv) $= 32.50\text{kN} \times 2.50\% = 0.73 \text{ kN/m}$

1. Horizontal force resulting from erection tolerances 1.0% of applied vertical load H, is 1& of 29.16kN $= 32.50\text{kN} \times 1.0\% = 0.33 \text{ kN/m}$

Total horizontal force= $0.73+0.33 = 1.06\text{N}$

Safe load of diagonal as strut. $= 1.06 / \cos 35 \text{ deg.} = 1.26\text{kN}$

Use least of A) coupler capacity of 6.30kN

B) Safe load of diagonal as strut.

6.30/1.26=50 say 4 standards, sobracing shall be provided every fourth standard at both direction.

Λ.E.